

# **Appendix D1**









TABLE D1-4  
UCSC Water Quality Monitoring Results & Statistical Analysis  
College 9/10 Spring

Analysis	Analytical Results***															Statistical Analysis***				California Toxics Rule <sup>1</sup>		Central Coast Basin Objectives <sup>2</sup>	Nat'l WQ Criteria <sup>3</sup>	
	1989-1990	1991-1992	1993-1994	1994-1995	1995-1996	1996-1997	1997-1998	1998-1999	1999-2000	2000-2001	2001-2002	2002-2003	2003-2004	2004-2005	Average	Maximum	Minimum	Std Dev	CMC <sup>4</sup>	CCC <sup>5</sup>		CMC	CCC	
pH value	No Analysis for 1989-90	6.1	6.1	6	6.6	6.4	6.8	6.8	6.9	6.3	6.8	6.6	6.3	6.6	6.4846	6.9000	6.0000	0.3051			7.0 - 8.5			
Conductivity (micromhos/cm)		290	260	130	170	140	140	235	210	210	180	210	180	190	195.7692	290.0000	130.0000	47.3395			750			
Carbonate Alk. (as CaCO3)		0	0	0	0	0	0	0	0	0	0	0	0	0	0.0000	0.0000	0.0000	0.0000						
Bicarbonate Alk. (as CaCO3)		38	35	21	26	32	38	38	39	18	26	36	35	38	32.3077	39.0000	18.0000	7.1576						
Total Alkalinity (as CaCO3)		38	35	21	26	32	38	38	39	18	26	36	35	38	32.3077	39.0000	18.0000	7.1576					200	
Total Hardness (as CaCO3)		100	75	36	56	44	38	58	59	59	51	59	51	56	57.0769	100.0000	36.0000	16.4086						
Total Dissolved Solids		190	170	88	110	90	90	150	150	140	120	140	120	120	129.0769	190.0000	88.0000	31.4284			250			
Nitrate (as NO3)		0.9	2.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	1.5	0.5	0.5	0.7615	2.5000	0.5000	0.5966			5*		10	
Chloride (Cl)		32	31	21	26	18	18	35	29	32	33	36	27	26	28.0000	36.0000	18.0000	6.0415			30	860	230	
Sulfate (SO4)		40	39	11	14	8.5	4.5	19	18	18	21	19	16	13	18.5385	40.0000	4.5000	10.4032			60			
Flouride (F)		0.2	0.05	0.05	0.05	0.1	0.1	0.1	0.11	0.26	0.05	0.05	0.05	0.05	0.0938	0.2600	0.0500	0.0863			1.0			
Calcium (Ca)		25	20	10	16	13	12	15	14	14	12	14	12	14	14.6923	25.0000	10.0000	3.9240						
Magnesium (Mg)		9.2	5.9	2.7	4	3	2	5.1	5.7	5.8	5.3	5.9	5	5.2	4.9846	9.2000	2.0000	1.8216						
Potassium (K)		1.9	2.3	1.3	3.3	1.5	1	1.7	1.8	1.9	2	3.3	1.7	2.9	2.0462	3.3000	1.0000	0.7207						
Sodium (Na)		16	20	11	14	12	14	15	20	18	15	20	16	19	16.1538	20.0000	11.0000	3.0509			25			
Total Iron (Fe)		1.3	0.33	1.65	0.88	1.15	0.57	0.19	0.3	1.2	0.43	0.71	4.9	0.78	1.1069	4.9000	0.1900	1.2209					0.300	
Manganese (Mn)		0.27	0.06	0.015	0.13	0.035	0.03	0.0075	0.02	0.11	0.022	0.02	0.16	0.038	0.0706	0.2700	0.0075	0.0772						
Arsenic (As)		0.005	0.0025	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.001	0.0038	0.001	0.0016	0.0050	0.0010	0.0013	0.34	0.15		0.34	0.15	
Barium (Ba)		0.05	0.16	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.0585	0.1600	0.0500	0.0305						
Boron (B)		0.025	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.0481	0.0500	0.0250	0.0069			0.2			
Cadmium (Cd)		0.0005	0.001	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0010	0.0005	0.0001	0.00135	0.00100		0.00135	0.00100	
Chromium (Cr)		0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.0037	0.001	0.0046	0.0050	0.0010	0.0011	0.23786	0.07710		0.23786	0.07710	
Copper (Cu)		0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.0250	0.0250	0.0250	0.0000	0.0051	0.0037	0.050	0.0051	0.0037	
Cyanide (CN)		0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.025	0.025	0.025	0.025	0.025	0.025	0.0169	0.0250	0.0100	0.0078	0.022	0.0052	0.05	0.022	0.0052	
Lead (Pb)		0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0000	0.0176	0.0069		0.0176	0.0069	
Mercury (Hg)		0.0001	0.0001	0.0001	0.0001	0.0005	0.0005	0.0005	0.0001	0.0001	0.0001	0.0005	0.0001	0.0001	0.0002	0.0005	0.0001	0.0002				0.0014	0.0007	
Selenium (Se)		0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0025	0.0000		0.005			0.005	
Silver (Ag)		0.0025	0.0025	0.0025	0.0025	0.005	0.005	0.005	0.0025	0.0025	0.005	0.005	0.005	0.005	0.0038	0.0050	0.0025	0.0013	0.00060			0.00060		
Zinc (Zn)		0.025	0.025	0.025	0.025	0.025	0.053	0.025	0.025	0.025	0.025	0.025	0.025	0.025	0.0272	0.0530	0.0250	0.0078	0.049	0.050		0.049	0.050	
MBAS (Surfactants)		0.01	0.025	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.0125	0.0125	0.0115	0.0250	0.0100	0.0042			0.2			
Aluminum (Al)		0.5	0.61	0.7	2.1	0.05	0.05	0.05	0.25	0.16	0.2	0.16	1.5	0.11	0.4954	2.1000	0.0500	0.6295			5.0	0.750	0.087	
Antimony (sb)		NR	NR	NR	NR	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.003	0.0030	0.0030	0.0030	0.0000						
Beryllium (Be)		NR	NR	NR	NR	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0005	0.0000			0.1			
Nickel (Ni)		NR	NR	NR	NR	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.005	0.0050	0.0050	0.0050	0.0000	0.197	0.022		0.197	0.022	
Thallium (Tl)		NR	NR	NR	NR	0.001	0.001	0.001	0.001	0.0005	0.0005	0.0005	0.0005	0.0005	0.0007	0.0010	0.0005	0.0003						
Nitrite (as NO2)		NR	NR	NR	NR	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.25	0.2500	0.2500	0.2500	0.0000			5			
Temperature		NR	NR	NR	NR	-	-	-	-	-	-	-	-	-	#DIV/0!	0.0000	0.0000	#DIV/0!						
Color (Co/Pt) (Units)		9	45	150	70	120	20	20	7	45	25	50	13	24	46.0000	150.0000	7.0000	43.9147			15	15		
Odor (Threshold Number)		0.5	1	0.5	2	0.5	0.5	0.5	0.5	0.5	0.5	3	0.5	0.5	0.8462	3.0000	0.5000	0.7742						
Turbidity (NTU)		4.2	18	29	68	26	4.0	4	2.4	1.9	2.5	2.7	12	4.9	13.8154	68.0000	1.9000	18.7624	5					
Petroleum Hydrocarbons (ppm)****		0.4	0.175	0.175	0.175	0.175	0.175	0.175	0.175	0.175	0.175	0.175	0.175	-	0.1938	0.4000	0.1750	0.0650			10**	0.1***		

**NOTES:**  
All units are in milligrams per liter, parts per million (ppm), unless expressed otherwise  
The highlighted columns are the columns for which the analytical results of WSW1 are compared against  
Criteria in pink are derived from "Parameters for Calculating Freshwater Dissolved Metals Criteria That Are Hardness-Dependent", and using the lowest non-zero hardness value taken from the data set  
<sup>1</sup> 40 CFR Part 131, Water Quality Standards; Establishment of Numeric Criteria for Priority Toxic Pollutants for the State of California; Rule [mg/L]  
<sup>2</sup> Central Coast Regional Water Quality Control Board Basin Plan Water Quality Objectives; Rule [mg/L]  
<sup>3</sup> US EPA National Recommended Water Quality Criteria; Rule [mg/L]  
<sup>4</sup> CMC<sup>4</sup> = Criteria Maximum Concentration - maximum concentration of a pollutant to which aquatic life can be exposed to for a short time without deleterious effects  
<sup>5</sup> CCC<sup>5</sup> = Criteria Continuous Concentration - maximum concentration of a pollutant to which aquatic life can be exposed to for 4 days without deleterious effects  
NR = Not Recorded  
NA = Not Analyzed; insufficient sample for analysis  
-- Not Analyzed during this sampling event  
\* Value taken from Table 3-3 "Water Quality Guidelines for Irrigation" for sensitive crops  
\*\* The CRWQCB Central Coast Region guidance value of <10 ppm for oil and grease, or <10,000 parts per billion (ppb)  
\*\*\* Value extracted from the Narrative Statement presented in the Red Book (EPA440/9-76-023, July, 1976) on "Oil and Grease" as harmful to aquatic animals  
\*\*\*\* Analytical results below detection limit are shown in green font at 1/2 the detection limit value, which is used in statistical analysis; Statistical analysis results in green font are derived entirely from 1/2 the analytical detection limit values  
\*\*\*\*\* Analytical results below detection limit are in green font: 1/2 the combined detection limit value for TPH diesel, kerosene & motor oil is used in statistical analysis















## **Appendix D2**

## Storm Water Runoff Analysis Methodology

The effects of increased impervious surfaces on storm water runoff were estimated using the US Army Corps of Engineers' (USACE) hydrologic modeling software program HEC-HMS. HEC-HMS is a hydrologic modeling system that provides several methods for analyzing rainfall runoff. Within HEC-HMS, the United States Soil Conservation Service (SCS) Curve Number method was used to determine how much of the precipitation would run off from each major watershed on campus. This is the same method used in the *Stormwater and Drainage Master Plan* for the modeling of the East Fork of Moore Creek (Kennedy/Jenks 2004). The SCS Curve Number method is an empirical method to calculate runoff from a rainfall event based on the soil type, land cover, and percentage of impervious surfaces. A runoff curve number (with higher values associated with increased runoff) is assigned based on the watershed properties. Inputs to the model include the rainfall data for a particular storm event and the watershed properties.

### Rainfall Event

Increasing the amount of impervious surfaces increases the percentage of rainfall that is converted to runoff. This effect tends to be more pronounced during smaller storms since in larger storms, as the ground becomes saturated, an increasing percentage of the precipitation becomes runoff. Two hypothetical precipitation events were chosen for this analysis to demonstrate how increasing the impervious area within the watershed affects runoff in a smaller storm and a larger storm. Specifically, precipitation events with 2-year and 25-year recurrence intervals were chosen for this analysis.

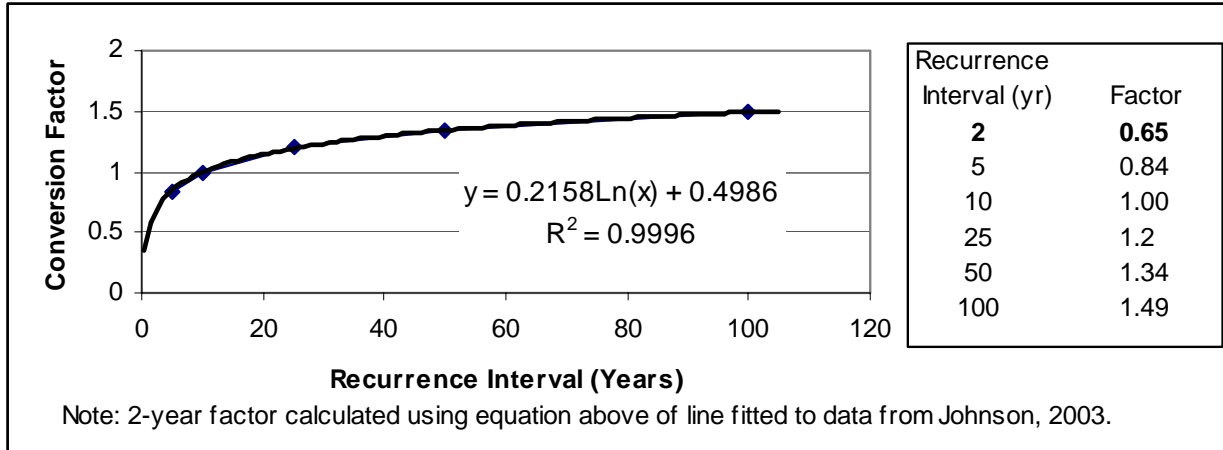
Graphs of intensity-duration curves show how the average intensity of rainfall over a storm event (in units such as inches per hour) decreases as the duration of the storm event increases. These curves vary by geographic location and by the frequency that a particular total volume of precipitation occurs during the storm duration. The Santa Cruz County Department of Public Works (SCCDPW) has published rainfall intensity-duration curves for events with a 10-year recurrence interval. The SCCDPW provides conversion factors to obtain the intensity for other storm events, including the 5-year, 25-year, 50-year, and 100-year. The 2-year conversion factor was determined through regression between the conversion factors and the storm recurrence interval, as shown in Figure D2-1. The 10-year frequency SCCDPW intensity-duration curves are given for different P60 values, or the rainfall intensity associated with a 100-year storm with a 60-minute duration. In Figure 9 of the *Evaluation of Drainage and Erosion Issues, Southeastern UC Santa Cruz Campus* (Johnson 2003), equations are provided for the SCCDPW 10-year intensity-duration curves. The P60 values at UC Santa Cruz are shown to range from approximately 1.5 to 1.9 inches per hour (in/hr) going from lower to higher elevations on campus. For this analysis, the 10-year frequency curve corresponding to a representative P60 value of 1.7 in/hr was chosen. Using  $P60 = 1.7$ , the equation for the average 10-year rainfall intensity,  $I_{10}$ , is:

$$I_{10} = 5.8931x^{-0.4015}$$

where  $x$  is the storm duration in minutes and  $I_{10}$  is in inches per hour (Johnson 2003)

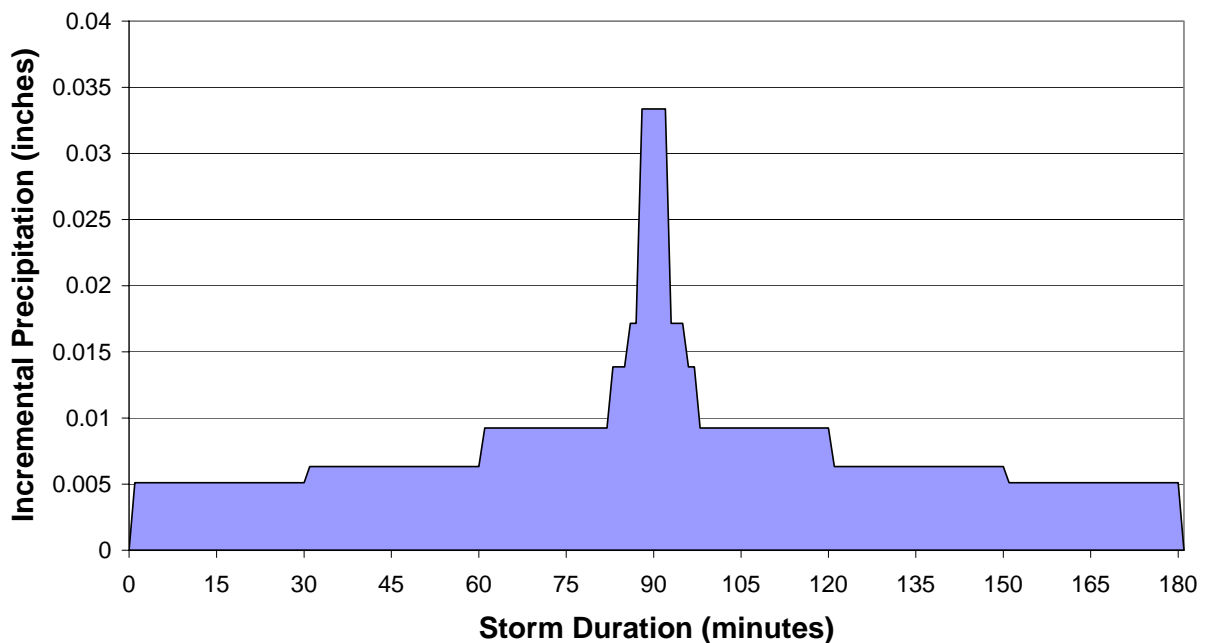
A duration for the storm events was also needed for the analysis. Although a 24-hour storm is often used for rainfall-runoff analyses for small watersheds, a shorter storm was determined to be more appropriate for analyzing the effects of the 2-year event. A shorter storm has a higher intensity, and so it provides a better measure of the maximum impacts associated with increasing the impervious surfaces. A 3-hour

storm was selected as this provides a storm long enough to have the entire watershed contributing to runoff. For the 25-year event, a 24-hour storm was chosen, as this duration is more commonly used in the design of storm control measures.



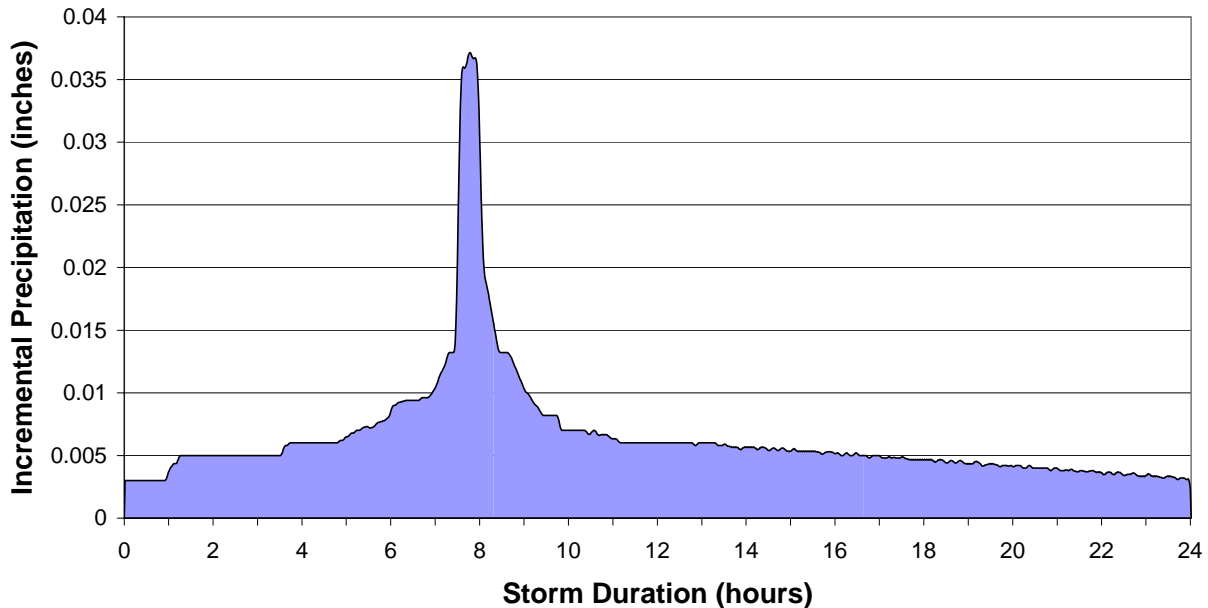
**Figure D2-1 Santa Cruz County Department of Public Works Conversion Factors for Other Recurrence Intervals**

By multiplying the 10-year intensity by the 2-year conversion factor, the 2-year, 3-hour rainfall intensity was determined to be 0.47 in/hr. This equates to a storm depth of 1.42 inches. A design storm was developed by assuming that the peak intensity occurs in the middle of the storm, and the remainder of the rainfall was distributed equally on both sides (see Figure D2-2).



**Figure D2-2 Rainfall Hyetograph for the 2-Year, 3-Hour Storm Event**

Using the conversion factor for the 25-year recurrence interval, the average rainfall intensity for the 25-year, 24-hour event was determined to be 0.38 in/hr. This equates to a storm depth of 9.15 inches. The SCS developed several rainfall distributions to represent how the rainfall intensity changes over the duration of a storm for different regions in the United States. UCSC is located within the region represented by the SCS Type 1a rainfall distribution. This storm distribution was used in HEC-HMS. The plot of the incremental precipitation over the duration of the storm based on the SCS Type 1a rainfall distribution is shown in Figure D2-3.



**Figure D2-3 Rainfall Hyetograph for the 25-Year, 24-Hour Storm Event**

#### Watershed Properties

The watershed properties used as input are shown in Table D2-1. The Wilder Creek and Moore Creek Western Tributary watersheds were not included in the model because there is no existing or planned development in these watersheds. The developed Employee Housing area South of Glen Coolidge Drive in the Arroyo Seco and High Street watersheds was included under existing conditions. It was also assumed that there would not be an increase in impervious area under the 2005 LRDP for the remaining portion of the High Street watershed.

The acreages of the existing and projected impervious area within each watershed were based on the LRDP land use maps showing the existing and proposed development areas. It was assumed that 60 percent of the existing developed areas were impervious, areas proposed for infill would increase the impervious area by 10 percent, and areas of new development would be 70 percent impervious. The total watershed area used as input to the HEC-HMS model was the same as the on-campus area, except for Cave Gulch, which used the total watershed area. This is because Cave Gulch is the only modeled watershed where the off-campus watershed area drains to the campus. It was assumed that the off-



campus portion of Cave Gulch was 20 percent impervious, which would be typical of residential areas with one-acre lots and represents the Cave Gulch rural residential area.

The runoff curve numbers used in the SCS Method were based on the hydrologic soil group of the various Natural Resources Conservation Service (NRCS), formerly known as SCS, soil types within each watershed and the type of vegetation cover – either meadow or woods in good hydrologic condition, meaning that the vegetation has not been disturbed so that the potential for capturing rainfall is not reduced. The area-weighted curve numbers are shown in Table D2-1 (with higher curve number values associated with more runoff).

The SCS method was used in HEC-HMS for the transformation of precipitation to runoff. This method was also used in the modeling of the East Fork of Moore Creek presented in the *Stormwater and Drainage Master Plan* (Kennedy/Jenks 2004). The input parameter required for this method is the lag time of each watershed. The lag time is equal to  $0.6 \times T_c$ , where  $T_c$  is the time of concentration in minutes. The time of concentration is defined as the time required for a drop of rainwater to flow from the hydrologically most remote point in the watershed to the basin outlet. The time of concentration was calculated using the empirically derived Kirpich Formula, which is the same formula that was used in the *Stormwater and Drainage Master Plan* (Kennedy/Jenks 2004):

$$T_c = 0.0078 L^{0.77} S^{-0.385}$$

where L is the length of the flow path in feet, and S is the average slope along the flow path. Table D2-2 shows the values used in the calculation of the lag time

**Table D2-1  
Existing and Projected On-Campus Impervious Area by Watershed**

<b>Watershed</b>	<b>Total Area (acres)</b>	<b>On Campus Watershed Area (acres)</b>	<b>Existing On-Campus Impervious Area (acres)</b>	<b>Projected Additional On-Campus Impervious Area (acres) Under 2005 LRDP</b>	<b>Percent Impervious for Existing Campus Watersheds</b>	<b>Percent Impervious for Campus Watersheds under the 2005 LRDP</b>	<b>Runoff Curve Number</b>
Wilder Creek	3,000	192	0	0	NA	NA	NA
Cave Gulch	460	336	7	54	7%	19%	67
Moore Creek	920	321	65	50	20%	36%	65
Moore Creek Western Tributary	320	98	0	0	NA	NA	NA
Jordan Gulch	1,380	440	90	54	21%	33%	64
Arroyo Seco	260	44	21	1	47%	49%	66
High Street	60	24	16	0	69%	69%	68
Kalkar Quarry	60	56	1	1	2%	3%	68
San Lorenzo River	74,000	509	42	58	8%	20%	61
<b>Total</b>		<b>2,020</b>	<b>243</b>	<b>218</b>	<b>13%</b>	<b>25%</b>	<b>64</b>

**TableD2-2**  
**UC Santa Cruz Campus Watershed Parameter Estimates for SCS Lag Time Calculation**

Watershed	Length of Flow Path (ft)	Highest Elevation (ft)	Lowest Elevation (ft)	Average Slope (ft/ft)	Time of Concentration (min)	Lag Time (min)
Moore Creek	9700	956	360	0.06	27	16
Cave Gulch	13000	1100	470	0.05	37	22
Jordan Gulch	12600	990	260	0.06	34	20
Arroyo Seco	2300	460	350	0.05	10	6
High Street	1800	380	280	0.06	8	5
Kalkar Quarry	3100	580	400	0.06	11	7
San Lorenzo River	11500	1160	700	0.04	36	22

Results of Analysis

The output from the HEC-HMS model provides the amount of rainfall that could potentially run off of each watershed under existing conditions and conditions under the 2005 LRDP for the two modeled storm events. The assumptions used for the model input will most likely overestimate the quantity of runoff because the volume of water draining through sinkholes or captured in depressions was not accounted for. It is difficult to quantify the amount of these losses because the amount draining to the subsurface depends on the infiltration rate of the sinkholes, which has not been studied in detail, and which also can change due to clogging. Also, it has been assumed that all the impervious areas within each watershed are directly connected and will contribute to runoff from the watershed. However, these results are useful on a planning level to assist in evaluating potential impacts due to the implementation of the 2005 LRDP by providing quantitative estimates of the maximum increase in runoff resulting from the proposed development. Results for the 2-year storm event are shown in Table D2-3. Results for the 25-year event are shown in Table D2-4.

**Table D2-3**  
**Results for a 2-Year, 3-Hour Storm Event Under Existing and Projected LRDP Conditions**

Watershed	Watershed Area Input to HEC-HMS (acres)	Cumulative Total Depth of Input Rainfall (inches)	Percentage of Rainfall as Runoff		Peak Flow (cfs)		Volume of Runoff (acre-feet)	
			Existing	2005 LRDP	Existing	2005 LRDP	Existing	2005 LRDP
Cave Gulch	460	1.42	28%	38%	122	161	16	21
Moore Creek	321	1.42	37%	49%	125	168	14	19
Jordan Gulch	440	1.42	37%	47%	158	198	19	25
Arroyo Seco	44	1.42	58%	60%	40	41	3	3
High Street	24	1.42	76%	76%	30	30	2	2
Kalkar Quarry	56	1.42	25%	26%	22	22	2	2
San Lorenzo River	509	1.42	25%	35%	119	165	15	21
<b>Total</b>	<b>1854</b>						<b>71</b>	<b>93</b>

**Table D2-4**  
**Results for a 25-Year, 24-Hour Storm Event Under Existing and Projected LRDP Conditions**

Watershed	Watershed Area Input to HEC-HMS (acres)	Cumulative Total Depth of Input Rainfall (inches)	Percentage of Rainfall as Runoff		Peak Flow (cfs)		Volume of Runoff (acre-feet)	
			Existing	2005 LRDP	Existing	2005 LRDP	Existing	2005 LRDP
Cave Gulch	460	9.2	68%	72%	586	622	239	254
Moore Creek	321	9.2	70%	76%	453	492	172	187
Jordan Gulch	440	9.2	70%	74%	587	626	236	251
Arroyo Seco	44	9.2	81%	82%	80	80	27	28
High Street	24	9.2	90%	90%	47	47	16	16
Kalkar Quarry	56	9.2	67%	68%	85	85	29	29
San Lorenzo River	509	9.2	63%	67%	586	634	244	263
<b>Total</b>	<b>1854</b>						<b>964</b>	<b>1028</b>

## References

Johnson, N.M. 2003. *Evaluation of Drainage and Erosion Issues, Southeastern UC Santa Cruz Campus*. June.

Kennedy/Jenks Consultants. 2004. *Stormwater & Drainage Master Plan*. September.

## Storm Water Runoff Analysis Methodology

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### Rainfall Event

Increasing the amount of impervious surfaces increases the percentage of rainfall that is converted to runoff. This effect tends to be more pronounced during smaller storms since in larger storms, as the ground becomes saturated, an increasing percentage of the precipitation becomes runoff. Two hypothetical precipitation events were chosen for this analysis to demonstrate how increasing the impervious area within the watershed affects runoff in a smaller storm and a larger storm. Specifically, precipitation events with 2-year and 25-year recurrence intervals were chosen for this analysis.

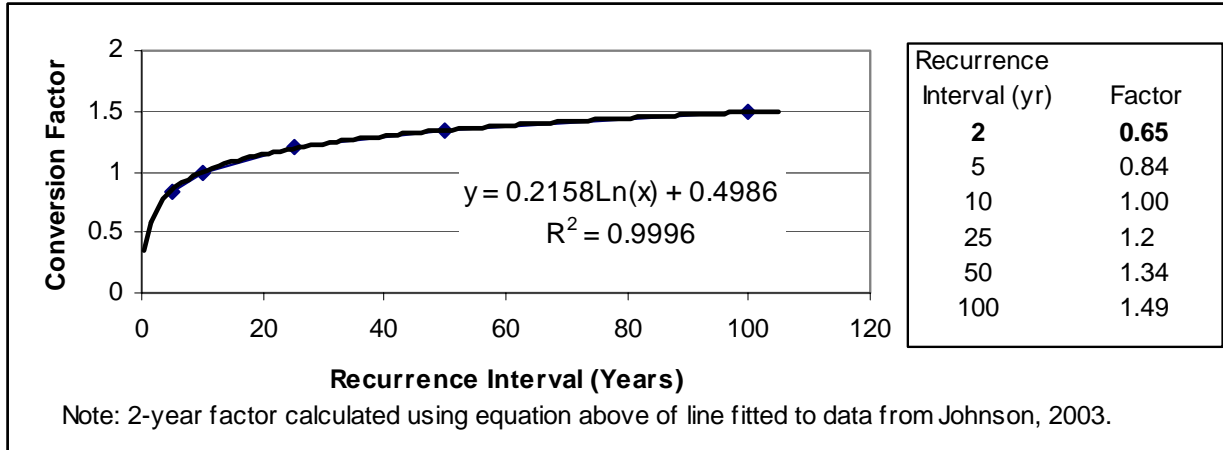
Graphs of intensity-duration curves show how the average intensity of rainfall over a storm event (in units such as inches per hour) decreases as the duration of the storm event increases. These curves vary by geographic location and by the frequency that a particular total volume of precipitation occurs during the storm duration. The Santa Cruz County Department of Public Works (SCCDPW) has published rainfall intensity-duration curves for events with a 10-year recurrence interval. The SCCDPW provides conversion factors to obtain the intensity for other storm events, including the 5-year, 25-year, 50-year, and 100-year. The 2-year conversion factor was determined through regression between the conversion factors and the storm recurrence interval, as shown in Figure D2-1. The 10-year frequency SCCDPW intensity-duration curves are given for different P60 values, or the rainfall intensity associated with a 100-year storm with a 60-minute duration. In Figure 9 of the *Evaluation of Drainage and Erosion Issues, Southeastern UC Santa Cruz Campus* (Johnson 2003), equations are provided for the SCCDPW 10-year intensity-duration curves. The P60 values at UC Santa Cruz are shown to range from approximately 1.5 to 1.9 inches per hour (in/hr) going from lower to higher elevations on campus. For this analysis, the 10-year frequency curve corresponding to a representative P60 value of 1.7 in/hr was chosen. Using  $P60 = 1.7$ , the equation for the average 10-year rainfall intensity,  $I_{10}$ , is:

$$I_{10} = 5.8931x^{-0.4015}$$

where  $x$  is the storm duration in minutes and  $I_{10}$  is in inches per hour (Johnson 2003)

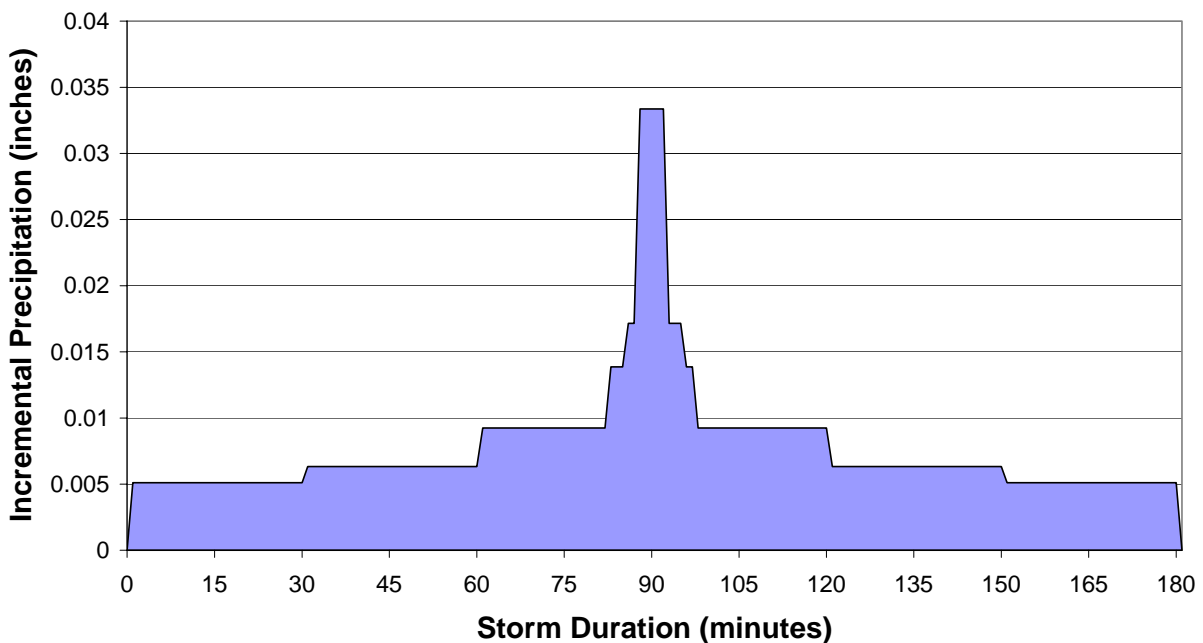
A duration for the storm events was also needed for the analysis. Although a 24-hour storm is often used for rainfall-runoff analyses for small watersheds, a shorter storm was determined to be more appropriate for analyzing the effects of the 2-year event. A shorter storm has a higher intensity, and so it provides a better measure of the maximum impacts associated with increasing the impervious surfaces. A 3-hour

storm was selected as this provides a storm long enough to have the entire watershed contributing to runoff. For the 25-year event, a 24-hour storm was chosen, as this duration is more commonly used in the design of storm control measures.



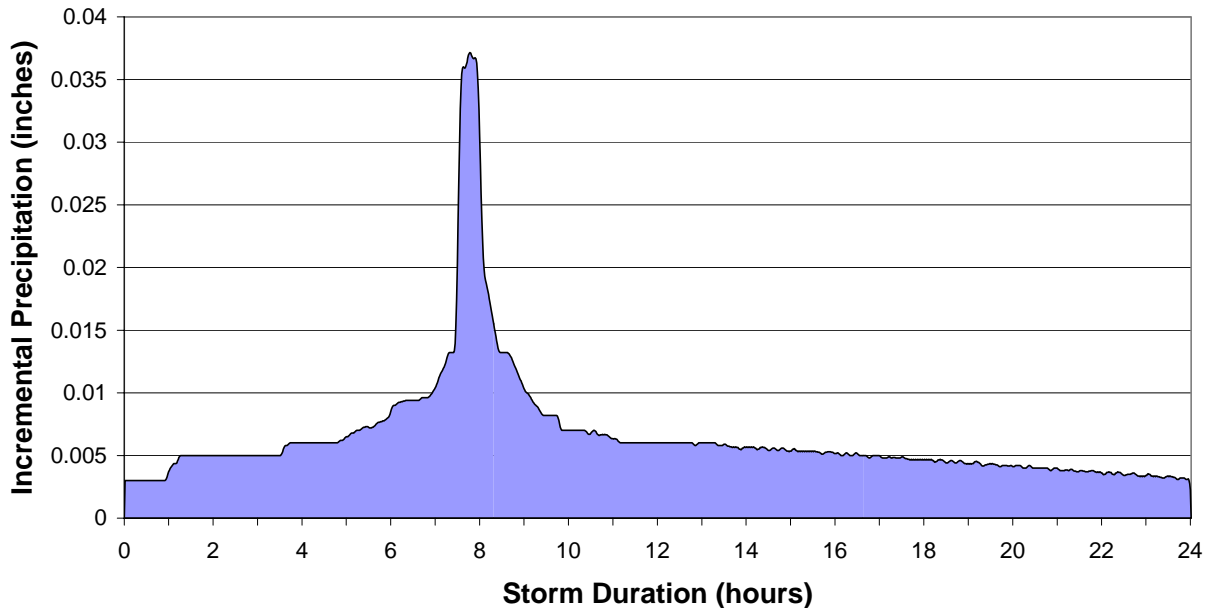
**Figure D2-1 Santa Cruz County Department of Public Works Conversion Factors for Other Recurrence Intervals**

By multiplying the 10-year intensity by the 2-year conversion factor, the 2-year, 3-hour rainfall intensity was determined to be 0.47 in/hr. This equates to a storm depth of 1.42 inches. A design storm was developed by assuming that the peak intensity occurs in the middle of the storm, and the remainder of the rainfall was distributed equally on both sides (see Figure D2-2).



**Figure D2-2 Rainfall Hyetograph for the 2-Year, 3-Hour Storm Event**

Using the conversion factor for the 25-year recurrence interval, the average rainfall intensity for the 25-year, 24-hour event was determined to be 0.38 in/hr. This equates to a storm depth of 9.15 inches. The SCS developed several rainfall distributions to represent how the rainfall intensity changes over the duration of a storm for different regions in the United States. UCSC is located within the region represented by the SCS Type 1a rainfall distribution. This storm distribution was used in HEC-HMS. The plot of the incremental precipitation over the duration of the storm based on the SCS Type 1a rainfall distribution is shown in Figure D2-3.



**Figure D2-3 Rainfall Hyetograph for the 25-Year, 24-Hour Storm Event**

#### Watershed Properties

The watershed properties used as input are shown in Table D2-1. The Wilder Creek and Moore Creek Western Tributary watersheds were not included in the model because there is no existing or planned development in these watersheds. The developed Employee Housing area South of Glen Coolidge Drive in the Arroyo Seco and High Street watersheds was included under existing conditions. It was also assumed that there would not be an increase in impervious area under the 2005 LRDP for the remaining portion of the High Street watershed.

The acreages of the existing and projected impervious area within each watershed were based on the LRDP land use maps showing the existing and proposed development areas. It was assumed that 60 percent of the existing developed areas were impervious, areas proposed for infill would increase the impervious area by 10 percent, and areas of new development would be 70 percent impervious. The total watershed area used as input to the HEC-HMS model was the same as the on-campus area, except for Cave Gulch, which used the total watershed area. This is because Cave Gulch is the only modeled watershed where the off-campus watershed area drains to the campus. It was assumed that the off-

campus portion of Cave Gulch was 20 percent impervious, which would be typical of residential areas with one-acre lots and represents the Cave Gulch rural residential area.

The runoff curve numbers used in the SCS Method were based on the hydrologic soil group of the various Natural Resources Conservation Service (NRCS), formerly known as SCS, soil types within each watershed and the type of vegetation cover – either meadow or woods in good hydrologic condition, meaning that the vegetation has not been disturbed so that the potential for capturing rainfall is not reduced. The area-weighted curve numbers are shown in Table D2-1 (with higher curve number values associated with more runoff).

The SCS method was used in HEC-HMS for the transformation of precipitation to runoff. This method was also used in the modeling of the East Fork of Moore Creek presented in the *Stormwater and Drainage Master Plan* (Kennedy/Jenks 2004). The input parameter required for this method is the lag time of each watershed. The lag time is equal to  $0.6 \times T_c$ , where  $T_c$  is the time of concentration in minutes. The time of concentration is defined as the time required for a drop of rainwater to flow from the hydrologically most remote point in the watershed to the basin outlet. The time of concentration was calculated using the empirically derived Kirpich Formula, which is the same formula that was used in the *Stormwater and Drainage Master Plan* (Kennedy/Jenks 2004):

$$T_c = 0.0078 L^{0.77} S^{-0.385}$$

where L is the length of the flow path in feet, and S is the average slope along the flow path. Table D2-2 shows the values used in the calculation of the lag time

**Table D2-1  
Existing and Projected On-Campus Impervious Area by Watershed**

<b>Watershed</b>	<b>Total Area (acres)</b>	<b>On Campus Watershed Area (acres)</b>	<b>Existing On-Campus Impervious Area (acres)</b>	<b>Projected Additional On-Campus Impervious Area (acres) Under 2005 LRDP</b>	<b>Percent Impervious for Existing Campus Watersheds</b>	<b>Percent Impervious for Campus Watersheds under the 2005 LRDP</b>	<b>Runoff Curve Number</b>
Wilder Creek	3,000	192	0	0	NA	NA	NA
Cave Gulch	460	336	7	54	7%	19%	67
Moore Creek	920	321	65	50	20%	36%	65
Moore Creek Western Tributary	320	98	0	0	NA	NA	NA
Jordan Gulch	1,380	440	90	54	21%	33%	64
Arroyo Seco	260	44	21	1	47%	49%	66
High Street	60	24	16	0	69%	69%	68
Kalkar Quarry	60	56	1	1	2%	3%	68
San Lorenzo River	74,000	509	42	58	8%	20%	61
<b>Total</b>		<b>2,020</b>	<b>243</b>	<b>218</b>	<b>13%</b>	<b>25%</b>	<b>64</b>

**TableD2-2**  
**UC Santa Cruz Campus Watershed Parameter Estimates for SCS Lag Time Calculation**

<b>Watershed</b>	<b>Length of Flow Path (ft)</b>	<b>Highest Elevation (ft)</b>	<b>Lowest Elevation (ft)</b>	<b>Average Slope (ft/ft)</b>	<b>Time of Concentration (min)</b>	<b>Lag Time (min)</b>
Moore Creek	9700	956	360	0.06	27	16
Cave Gulch	13000	1100	470	0.05	37	22
Jordan Gulch	12600	990	260	0.06	34	20
Arroyo Seco	2300	460	350	0.05	10	6
High Street	1800	380	280	0.06	8	5
Kalkar Quarry	3100	580	400	0.06	11	7
San Lorenzo River	11500	1160	700	0.04	36	22

Results of Analysis

The output from the HEC-HMS model provides the amount of rainfall that could potentially run off of each watershed under existing conditions and conditions under the 2005 LRDP for the two modeled storm events. The assumptions used for the model input will most likely overestimate the quantity of runoff because the volume of water draining through sinkholes or captured in depressions was not accounted for. It is difficult to quantify the amount of these losses because the amount draining to the subsurface depends on the infiltration rate of the sinkholes, which has not been studied in detail, and which also can change due to clogging. Also, it has been assumed that all the impervious areas within each watershed are directly connected and will contribute to runoff from the watershed. However, these results are useful on a planning level to assist in evaluating potential impacts due to the implementation of the 2005 LRDP by providing quantitative estimates of the maximum increase in runoff resulting from the proposed development. Results for the 2-year storm event are shown in Table D2-3. Results for the 25-year event are shown in Table D2-4.

**Table D2-3**  
**Results for a 2-Year, 3-Hour Storm Event Under Existing and Projected LRDP Conditions**

<b>Watershed</b>	<b>Watershed Area Input to HEC-HMS (acres)</b>	<b>Cumulative Total Depth of Input Rainfall (inches)</b>	<b>Percentage of Rainfall as Runoff</b>		<b>Peak Flow (cfs)</b>		<b>Volume of Runoff (acre-feet)</b>	
			<b>Existing</b>	<b>2005 LRDP</b>	<b>Existing</b>	<b>2005 LRDP</b>	<b>Existing</b>	<b>2005 LRDP</b>
Cave Gulch	460	1.42	28%	38%	122	161	16	21
Moore Creek	321	1.42	37%	49%	125	168	14	19
Jordan Gulch	440	1.42	37%	47%	158	198	19	25
Arroyo Seco	44	1.42	58%	60%	40	41	3	3
High Street	24	1.42	76%	76%	30	30	2	2
Kalkar Quarry	56	1.42	25%	26%	22	22	2	2
San Lorenzo River	509	1.42	25%	35%	119	165	15	21
<b>Total</b>	<b>1854</b>						<b>71</b>	<b>93</b>



**Table D2-4**  
**Results for a 25-Year, 24-Hour Storm Event Under Existing and Projected LRDP Conditions**

Watershed	Watershed Area Input to HEC-HMS (acres)	Cumulative Total Depth of Input Rainfall (inches)	Percentage of Rainfall as Runoff		Peak Flow (cfs)		Volume of Runoff (acre-feet)	
			Existing	2005 LRDP	Existing	2005 LRDP	Existing	2005 LRDP
Cave Gulch	460	9.2	68%	72%	586	622	239	254
Moore Creek	321	9.2	70%	76%	453	492	172	187
Jordan Gulch	440	9.2	70%	74%	587	626	236	251
Arroyo Seco	44	9.2	81%	82%	80	80	27	28
High Street	24	9.2	90%	90%	47	47	16	16
Kalkar Quarry	56	9.2	67%	68%	85	85	29	29
San Lorenzo River	509	9.2	63%	67%	586	634	244	263
<b>Total</b>	<b>1854</b>						<b>964</b>	<b>1028</b>

## References

Johnson, N.M. 2003. *Evaluation of Drainage and Erosion Issues, Southeastern UC Santa Cruz Campus*. June.

Kennedy/Jenks Consultants. 2004. *Stormwater & Drainage Master Plan*. September.